

Public Works Department

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Stowe, VT 05672
802-253-8770

MEMORANDUM

TO: Charles Safford, Town Manager
CC: Selectboard
FROM: Harry Shepard, PE, Public Works Director/Town Engineer
DATE: November 21, 2024
SUBJECT: Stowe Water and Sewer Capacity Evaluation

As requested, this Memorandum is intended to answer the question asked by the Selectboard/Water and Sewer Commissions and an increasing number of interested Board/Commissions, Developers, and members of the Public; “What is the Capacity of Stowe’s Water and Sewer systems? This effort has included significant data accumulation and analysis, review and analysis of related permits, consultation with our Engineering consultant and VTDEC regulators, and consideration of the physical size, location and operating conditions of various water and sewer system components.

First, I would like to define “Capacity” and clarify the focus of this review. Sub-Chapter 21-2 of the Vermont Water Supply Rules define Capacity to mean “*that a public water system has the technical, financial, and management capabilities to consistently comply with current performance standards, including the requirements of this rule and the Safe Drinking Water Act, 42 U.S.C Section 300f as amended.*” The requirements for “technical (sometime referred to as operational), financial and management capabilities” are common tenets for defining Utility capacities. This effort is focused solely on the Town’s technical capacities.

Second, it will be helpful to understand the capacity considerations to follow by having a general understanding of the historical development and extent of the Town’s water and sewer systems. These are as follows:

Sewer: The original Stowe sewer system consisted of a wastewater treatment plant (WWTP), designed for 0.165 million gallons per day (MGD), with a service area and collection system that included the former Stowe Village and Lower Village and the Lower Village Sewer Pump Station (LVPS), constructed in the late 1970’s. Some elements of this original collection system are remnants of clay tile sewers associated with abandoned common leaching systems or direct point source discharges to the North and West Branch’s and the Little River. In the mid-1990s, the WWTP’s capacity was increased to 0.225 MGD by the construction of the headworks building and sludge storage bunker.

In the late 1990’s-early 2000’s, major expansions of the sewer service area and collection system with two additional pump stations - Weeks Hill and Mansfield Base were developed. The WWTP was expanded to increase its permitted capacity to 1.0 MGD and upgraded to increase the quality of the effluent discharge and residuals to produce a Class A biosolid. In the mid 2000’s, in response to odor control issues, foul air improvements were implemented. In 2018, in response to continued odor challenges, the use of the residuals processing system (ATAD) used to produce Class A biosolid was suspended and odor issues subsided. Exhibit A attached is a plan that depicts key elements of the Stowe Sewer System.

Water: Stowe's water system was established by the former Village in 1904. The original system served the core of the Village and was supplied by various spring sources proximate to the original reservoir on Sunset Hill. These sources proved inadequate during dry weather periods and in 1934, the Village purchased a 10 Acre parcel of land with more dependable spring water sources on Edson Hill (Robinson Springs) and installed a 6" cast iron transmission pipe from the source to the Sunset Hill reservoir. For 65 years, this source dependably served the Village without treatment or pumping. This source remains one of the Town's two water supplies. Upgrades for minimal treatment (pH adjustment and disinfection) have occurred and the supply is permitted for 213 gallons per minute (GPM). The 6" cast iron transmission pipe (now distribution), which is 90 years old, also remains in service along the upper portions of Edson Hill Road.

In the late 1990's and early 2000's, concurrent with the major expansions of the sewer system and intent to similarly expand the water system, an additional source of supply was required. Finding and developing this source of supply was a major challenge for the Town during this period and our second source, our Village Green facility, was secured, permitted, and developed. The Village Green facility lays above a classic "confined aquifer" (water bearing soil layer below a fine grained (low permeability) soil overburden that provides some level of protection of the underlying water quality from surface influences). This source has a known physical yield capacity estimated to be over 1000 GPM. This raw water source is high in Iron and Manganese. This facility is currently permitted for 413 GPM with a treatment plant that includes filtration for iron and manganese, and disinfection. The plant was designed for and constructed to be expanded for additional future capacity, with provisions for an additional filter train. A redundant well, installed in 2015, was installed, and developed, but not permitted, to increase the facilities supply capacity to 1000 GPM+/- . The water distribution system and service area were updated and expanded and now includes 28+/- miles of distribution piping with 9 water storage tanks and 11 booster pump stations serving 15 pressure zones, each with their own capacity characteristics. Some of these distribution elements were former private water systems now supplied by and incorporated into the Town's distribution system. Exhibit B attached depicts key elements of the Stowe Water System.

Third, for discussions of capacity below, our sewer system will be subdivided into our WWTP, and Collection System and our water system will be subdivided into our Sources/Treatment and Distribution System. These are as follows:

Sewer-WWTP: Stowe is fortunate to have a relatively modern WWTP operating under VTDEC Discharge Permit No 3-1232, with a hydraulic capacity of 1.0 MGD Average Daily Flow. Our Average Daily Flow in 2023 was 0.368 MGD but as a resort community, we have a large seasonal variation in flows, typically ranging from 0.3-0.5 MGD+/- Our Peak Daily Flow, typically associated with high water-wet weather events, was approximately 0.8 MGD. This occurred during the July 2024 flood event. The influent contribution from a typical residence varies from 150 to 165 GPD. Hydraulically, our WWTP has adequate capacity for the foreseeable future.

Our WWTP also has organic discharge limitations associated with our Discharge Permit. Exhibit C attached is a copy of a portion of our discharge monitoring report submitted monthly to VTDEC. This lists the various constituents monitored and reported for both concentrations and mass loading. Each represents a capacity consideration. As a resort community, most of our influent is domestic in nature, although some of our restaurants and breweries/cider makers produce a higher strength influent wastewater. As part of the commissioned efforts by our Engineering consultants associated with the 20-year review of our WWTP performance required by our Discharge Permit, each of these constituents were analyzed and affirmed to be well within the WWTP's original design criteria and we remain compliant current Discharge Permit limitations. Organically, our WWTP also has adequate capacity for the foreseeable future.

Sewer Collection System: In general, the collection system installed with the major expansions constructed in the early 2000's, including the Weeks Hill and Mansfield Pump Stations, are both hydraulically adequate and in good operating condition. In general, this portion of our system (above Weeks Hill Road) are deemed to have adequate capacity for the foreseeable future. One capacity limitation which is permit related is for properties along the Mountain Road in the Upper Mountain Road (UMR) zoning district (from Strom Farm Road to the base of Harlow Hill, except at the Edson Hill Road intersection). Our Act 250 permit for the WWTP and collection system expansion prohibits the allocation of sewer flows greater than the uses that existed on March 16, 2001. This was intended to limit development along this corridor and has often resulted in capacity limitations for these properties.

The original collection system (below Weeks Hill Road) is generally hydraulically adequate and in fair operating condition, but exceptions exist, most significantly, the Lower Village Sewer Pump Station (LVPS). This critical collection system element is 45 years old and antiquated. It performs admirably and within its design capacity of 396,000 GPD during normal dry weather flow conditions, but significantly exceeds its design capacity during high water-wet weather events. During the recent July 2024 event, it pumped an estimated 847,000 GPD. When these more frequent events occur, both lead and lag pumps operate simultaneously for extended continuous run times periods. This is indicative of capacity limitations. Either pump failure during these periods would likely result in a direct discharge of untreated sewage and a permit violation. The Town has completed the Preliminary Engineering design for the proposed replacement of the LVPS. In general, this will consist of a new Dry-Pit type facility abutting and south of the existing facility. Additional property acquisition will be required. We are also currently studying the adequacy of the existing sewer force main from the LVPS to the Headworks building, which includes a river crossing and upgrades are likely. We are currently targeting 2027-2028 for construction dependent on permits and funding. The current Estimated Total Cost is \$3-4M. Until this upgrade is implemented, I would classify our sewer capacity in this portion of our service area (Village and Lower Village) as capacity limited. Modest growth over the next few years can be accommodated but risks of pump failure during these more frequent wet weather events will remain high until upgrades are implemented.

Another capacity limitation in the original collection system is where remnants of original clay tile sewer pipe remain. These are antiquated and can be in poor condition. We typically address these by replacement, either by the Town or Developers looking to connect as a capacity concern. A recent example was the replacement of the common clay tile sewer behind the buildings on the north side of Main St. This was undertaken in phases by both the Town and Developers because of this localized capacity concern.

A final capacity limitation with the sewer collection system that should be noted is that significant portions of the sewer service area do not include any abutting existing collection system. Exhibit A includes a repeated notation, "Limits of Gravity Collection System" These are the ends of our existing collection system, usually limited from extension by existing grade conditions. In these situations, extensions of the collection system will require new sewer pump stations to have "Capacity to Serve".

Water-Sources/Treatment: A general description of our two sources of supply, treatment and permitted yields was previously described. Edson Hill is a high elevation source with better water quality requiring less treatment. It was also formally a naturally occurring artesian spring discharging approximately 180-200 GPM to the surface 24/7/365. During the efforts to expand the system in the early 2000's, extensive geo-hydraulic assessment to evaluate the potential for more water from this source was undertaken to no avail. It is a steady 190GPM+/- source, 24/7/365, but not more. The source protection area is relatively small and undeveloped except for a couple of large lot residences. Much of the Town's 10 Acre parcel is immediately upgradient of these wells in that Source Protection Area. We monitor the drawdown level in this well and have documented acceptable seasonal variation while maintaining an average withdrawal 186 GPM, +/-5 GPM, 24 hour/day. Operationally, we know that attempts to operate the well pumps at less than 24 hours/day or at withdrawal rates above 195 GPM create numerous operational challenges including loss of suction in the adjacent Edson Hill Manor hydro-pneumatic pump station and vacuum conditions that have damaged and altered the function downstream elements, including 2 pressure sustaining valves. This well is best operated 24/7/365 as we do.

Village Green has a high yield capacity “confined” aquifer but requires more treatment. It is also located in the Mountain Road Village zoning district, a growth center with commercial and residential uses and a golf course within its source protection area. This area is also experiencing other development growth pressures. It also has relatively low but detectible levels of PFAS, which calls into question the effectiveness of the confining layer above the underlying aquifer.

Operationally, we try to use as much water from Edson Hill as possible because of the reduced treatment requirements. We are fortunate to have a distribution system that allows this. We can provide water to all pressure zones except one (Stowe Club) from either source. Our Stowe Club pressure zone can only be supplied from Village Green. During peak demand periods in the winter season, most of Edson Hill water is used above Edson Hill Road, supplemented as required for demand by water from Village Green. During other seasons when the demand above Edson Hill Road is significantly less, more Edson Hill water flows by gravity toward the Village and is mixed with Village Green water to meet the demands of our pressure zones below Edson Hill Road. Although not officially blending, this does provide reduced levels of PFAS in most of our pressure zones.

Exhibit D attached is a graphic depicting a Water System Demand & Capacity Analysis. It depicts key information regarding our demand trends from 2010-2023 and existing permitted supply considerations. As a resort community, we also experienced higher seasonal demands and it also includes an estimate of our “Maximum Occupancy Average Day Demand”, which is another demand criteria in the Water Supply Rules. The horizontal lines depict individual capacities for both sources using our permitted yields for both 12 and 24 hours per day. It also includes how we operate Edson Hill at a slightly reduced yield of 186 GPM +/-, 24 hours per day and combinations for both sources. The Average Day Demand (ADD) is the total amount of water used by a system for one year divided by 365. The Maximum Day Demand (MDD) is the largest volume of water delivered to the system in a single day. In general, 12-hour withdrawals are used for capacity considerations associated with the ADD and 24-hour withdrawals are used for MDD.

Our ADD, which has been trending positive approximately +7% per year, started to exceed our combined 12-hour withdrawals in 2022 however, our ADD is still well below the combination of the 12-hour withdrawal from Village Green and our operational 24-hour withdrawal from Edson Hill. This also satisfies our current estimated Maximum Occupancy ADD. Section 2.2.1 of the Water Supply Rules require that if your ADD exceeds your 12-hour permitted withdrawals, “*the water system immediately apply for additional source capacity*”. This is one of the reasons why we proposed, and the Water Commission approved an FY23 Water Capital project to commence the Engineering design, testing and permitting to increase the permitted yield and treatment capacity of our Village Green facility. This effort has been suspended because of detectible PFAS in our Village Green supply. However, our efforts to achieve additional source capacity have not been suspended and if necessary, capacity expansion at our Village Green facility incorporating PFAS treatment may become necessary. A Preliminary Engineering Study to do so has been completed. VTDEC has affirmed in writing that for the purposes of source capacity considerations, the Water Supply Rule requirements for the MDD will control.

Our current MDD is 708,680 gallons which is 79% of our Max Day Capacity and 87% of our 90% Max Day Capacity, which is the threshold when the Water Supply Rule requires planning for additional sources, which we already have begun. Exhibit D includes approximations of existing reserve source capacity using the MDD requirements. At 90% of MDD, we estimate that we have capacity for an additional 700-1400 bedrooms. At 100% MDD, we have capacity for an estimated 1400 – 2800 bedrooms. The lower range is based upon allocation flows. The higher range is based upon estimates of actual flows, which are generally less than 1/2 of the allocation flows. Our existing sources have the capacity for modest growth.

Water-Distribution System: Because of our mountainous terrain and large service area (for a VT municipal system), our distribution system is complex. During peak demand periods in the winter season, water from Village Green is pumped 5 times and flows through 3 water storage tanks to reach our most remote customers.

Our Engineering consultant who specializes in VT Water and Wastewater commented that Stowe has the most complicated distributions system in Vermont. As noted previously, each pressure zone has its own unique capacity characteristic. Key elements related to capacity in the distribution systems include the following:

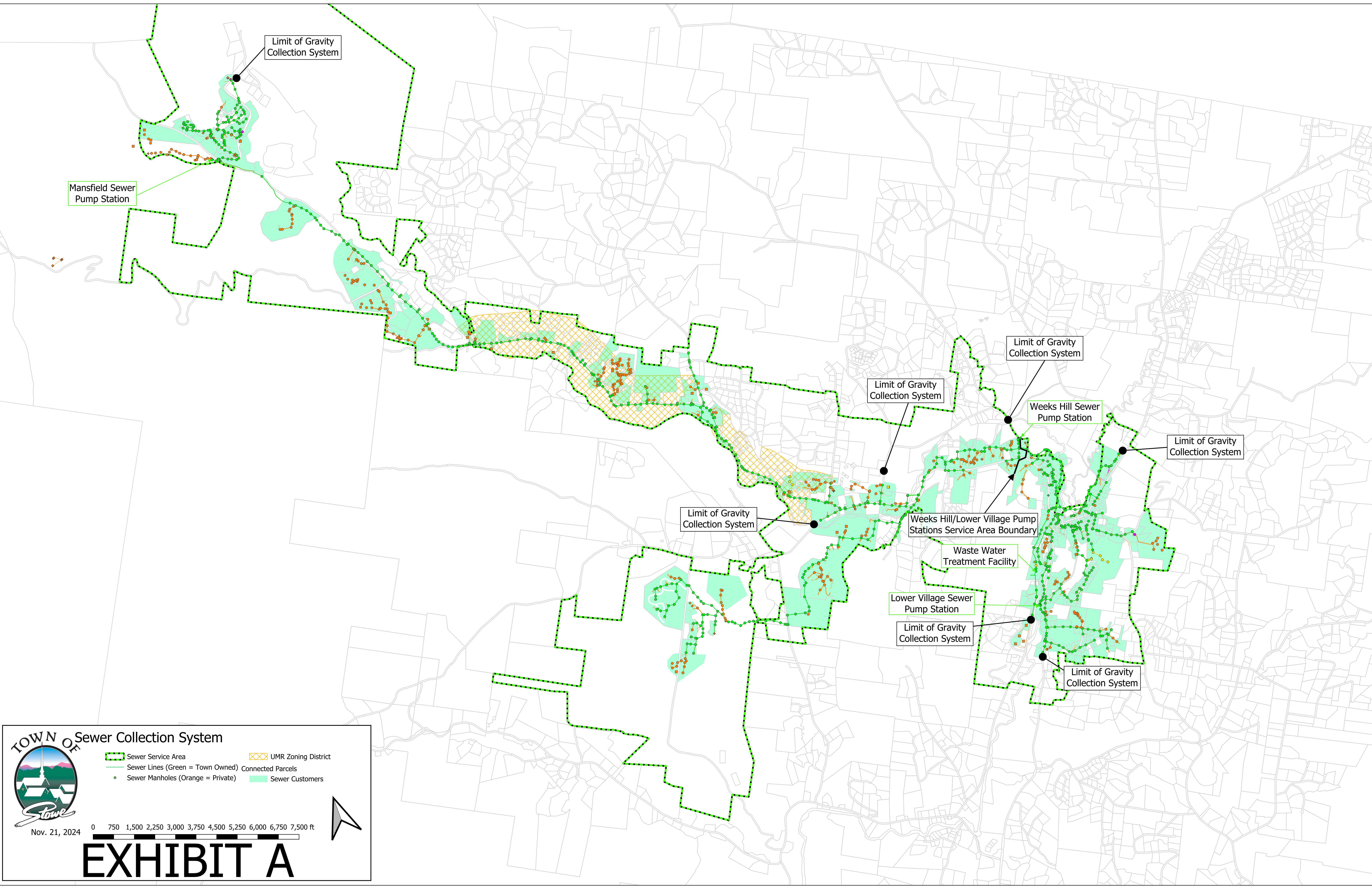
- Volume of Stored Water: At a minimum, water storage tanks must hold at least the ADD and if Fire Protection System are to be served, the ADD + 60,000 gallons minimum for Fire demand.
- If fire protection systems are served, the minimum size of the distribution piping must be 8" minimum.
- Elevation of Stored Water: This establishes the service pressures in each pressure zone. For our distribution system, which is generally linear and relatively simple hydraulically, we do not have the "Capacity to Serve" minimum service pressures required by the Water Supply Rules above elevations 100' +/- below the stored water elevations.

Exhibit E attached is a Water and Sewer Capacity Map. It depicts relevant general capacity information for our water and wastewater facilities, sewer service area and sub-areas to each of our sewer pump stations, water pump stations, storage tanks and pressure zones, approximate maximum service elevation for each pressure zone, and connected parcels with water and/or sewer service.


The column titled "Reserve ADD Capacity" under the Drinking Water Pressure Zone Capacity provides an estimate of additional capacity in each pressure zone. Sunset is our largest pressure zone with a 729,000-gallon storage tank and approximately 451,000 GPD of Reserve ADD capacity. The Springer-Miller project is in this pressure zone, and we have "Capacity to Serve". Excluding our hydro-pneumatic pressure zones, which serve a few higher elevation residences (and are capacity limited), our AIG pressure zone which serves the Toll House area is our most limited pressure zone with only 5000 GPD ADD reserve capacity. We have "Capacity to Serve" small projects such as adding bedrooms to existing buildings, like a few recent applications in the Mt. Mansfield Townhome Condominiums, but for any larger development proposal, we would not. This distribution system also has 6" maximum pipe so we would not have "Capacity to Serve" any project proposed in this pressure zone that required Fire Protection systems. The recent golf course development in the Stowe Club pressure zone would have Allocation Flows that exceed the Reserve ADD in that pressure zone. This is one of the reasons why I advised the developer that I would not recommend approval of Allocations to the Water and Sewer Commission for this project, because we would not have the "Capacity to Serve". We do not have a sewer collection proximate to Shaw's supermarket and this is one of the locations where a sewer pump station would be required to extend the existing collection system further north to provide service. Again, we currently do not have "Capacity to Serve".

The above is intended to provide recent examples of "Capacity to Serve" determinations that we regularly make before presenting Allocation recommendations to the Water and Sewer Commissions. In some instances, upgrades and/or expansions are negotiated with the developers to remedy Capacity to Serve concerns. Spruce Peak Realty constructed more water storage in our Spruce pressure zone. Percy's proposed extension of our water distribution system to serve their new subdivision off of Cape Cod Road. These are recent examples of developer sponsored upgrades to provide us with the Capacity to Serve. Shaw's supermarket decided against constructing a municipal sewer pump station and that project was not recommended for allocation because we lacked the Capacity to Serve.

I hope this adequately answers the question "What is Stowe Water and Sewer System Capacity? The answer is not simple, but neither is our systems. In general, we have capacity, but this is limited, and each project needs to be reviewed and considered as we currently do on a case-by-case basis as described herein. Given the growth that has occurred in the Town over the last 15 years, it will be prudent for the Town to advance the efforts for securing additional source capacity for the water system and replacement of the LVPS for the sewer system to have adequate capacity for the foreseeable future, These are both projects that Public Works is actively working to advance.





TOWN OF





Stowe


Sewer Collection System

 Sewer Service Area

 Sewer Lines (Green = Town Owned) Connected Parcels

 Sewer Manholes (Orange = Private)

 UMR Zoning District

 Sewer Customers

Nov. 21, 2024

07501,5002,2503,0003,7504,5005,2506,0006,7507,500

ft

EXHIBIT

A



Water Distribution Map

0 0.5 1 1.5 2 mi

0 1,000 2,000 3,000 4,000 5,000 6,000 7,000 8,000 9,000 10,000 ft

Updated Nov. 21, 2024

EXHIBIT B

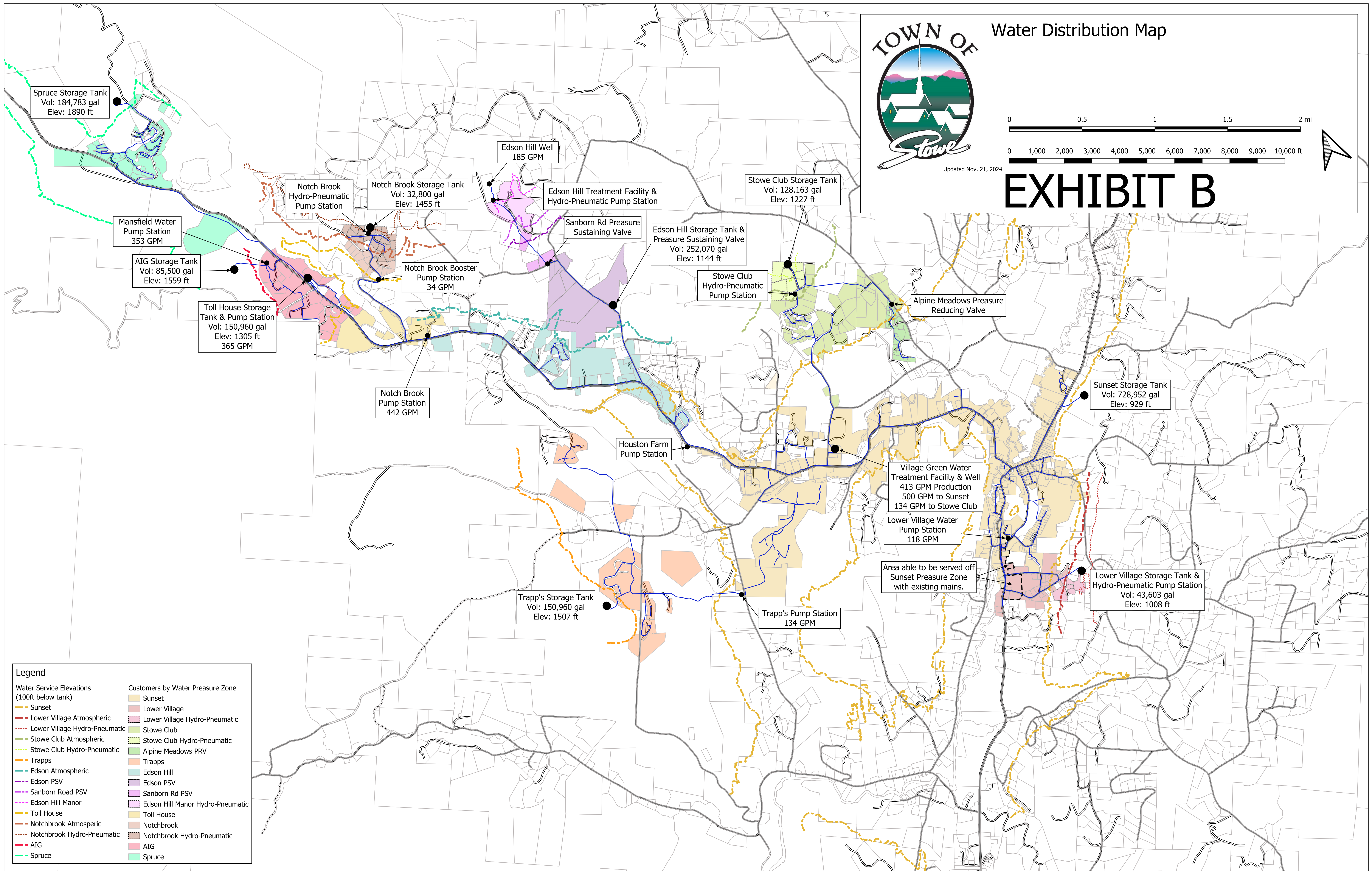


EXHIBIT C

VERMONT WASTEWATER PERMIT PROGRAM

Permittee: Town of Stowe - Permit # 3-1232

Month: Jul-24

PERMIT MONITORING INFORMATION FOUR BASIN SBR FACILITY

DATE	INFLUENT		EFFLUENT																						Effluent Primary Flow Device Is A 90 Degree V-notch Weir							
	BOD	TSS	BOD		TSS		TKN		UOD	Total Phosphorus		Total Ammonia		Total Nitrogen		NOx		Total Copper		E-Coli	Settleable Solids	pH	Turbidity	UV Intensity	Total Daily Flow	Effluent Flow Checks						
	mg/L	mg/L	mg/L	lbs	mg/L	lbs	mg/L	lbs	lbs	mg/L	lbs	mg/L	lbs	mg/L	lbs	mg/L	lbs	mg/L	lbs	#/ 100 ml	ml/L	su	ntu	%	mgd	Date	Head	Actual Flow	Chart Flow	(Actual - Chart) x 100 = % error		
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	Actual		28
1																				0.0	7.05	0.15	100	0.400	7/3/2024	0.563	0.384	0.379	1.4	% error		
2																				0.0	7.14	0.12	100	0.415	7/10/2024	0.708	0.682	0.662	2.9	% error		
3	190	222	1.30	3.77	1.00	2.90	0.60	1.74	13.4	0.18	0.52	0.10	0.29	3.80	11.03	3.20	9.29	0.003	0.007	1	0.0	7.15	0.12	100	0.348	7/17/2024	0.396	0.159	0.164	-2.8	% error	
4																				0.0	7.04	0.14	100	0.426	7/24/2024	0.458	0.229	0.237	-3.3	% error		
5																				0.0	7.01	0.14	100	0.378						% error		
6																				0.0	6.96	0.16	100	0.491	Factory Calibration Date:			By:				
7																				0.0	6.94	0.16	100	0.397	Comments And Explanations Of Any Violations: (Reference all attachments here)							
8																				0.0	7.07	0.17	100	0.313								
9																				0.0	7.06	0.13	100	0.335								
10																				0.0	7.06	0.13	100	0.421								
11	130	180	1.30	8.68	1.00	6.68	0.80	5.34	36.8	0.24	1.60	0.22	1.47	3.90	26.05	3.10	20.71	0.003	0.019	1	0.0	7.11	0.23	100	0.801							
12																				0.0	7.04	0.13	100	0.542								
13																				0.0	6.90	0.11	100	0.558								
14																				0.0	7.01	0.12	100	0.427								
15																				0.0	7.02	0.12	100	0.462								
16																				0.0	7.05	0.12	100	0.395								
17																				0.0	7.02	0.12	100	0.452								
18	220	172	1.20	4.41	1.00	3.68	0.70	2.57	18.1	0.17	0.63	0.10	0.37	3.70	13.61	3.00	11.03	0.002	0.008	1	0.0	7.01	0.13	100	0.441							
19																				0.0	7.03	0.12	100	0.398								
20																				0.0	7.02	0.13	100	0.419								
21																				0.0	7.00	0.12	100	0.350								
22																				0.0	7.09	0.15	100	0.350								
23																				0.0	7.14	0.16	100	0.369								
24																				0.0	7.13	0.14	100	0.370	Prepared By:			Bryan Longe				
25	210	206	5.30	16.13	1.00	3.04	0.70	2.13	32.8	0.24	0.73	0.50	1.52	4.10	12.48	3.40	10.35	0.003	0.009	1	0.0	7.05	0.16	100	0.365							
26																				0.0	7.01	0.17	100	0.383								
27																				0.0	7.04	0.13	100	0.378	I certify under penalty of law that I have personally examined and am familiar with the information submitted herein. Based on my inquiry of those individuals immediately responsible for obtaining the information, I believe the submitted information is true, accurate and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment.							
28																				0.0	7.04	0.14	100	0.370								
29																				0.0	7.18	0.12	100	0.369								
30																				0.0	7.04	0.14	100	0.333								
31																				0.0	7.02	0.13	100									
Total	750	780	9.10	33.00	4.00	16.30	2.80	11.79	101.1	0.83	3.48	0.92	3.65	15.50	63.17	12.70	51.38	0.010	0.042	4			4.30	3100	12.456	Approved By:						
Ave.	188	195	2.28	8.25	1.00	4.08	0.70	2.95	25.3	0.21	0.87	0.23	0.91	3.88	15.79	3.18	12.85	0.003	0.011	1			0.14	100	0.415							
Max.	220	222	5.30	16.13	1.00	6.68	0.80	5.34	36.8	0.24	1.60	0.50	1.52	4.10	26.05	3.40	20.71	0.003	0.019	1		7.18	0.23	100	0.801							
Min.	130	172	1.20	3.77	1.00	2.90	0.60	1.74	13.4	0.17	0.52	0.10	0.29	3.70	11.03	3.00	9.29	0.002	0.007	1		6.90	0.11	100	0.313							
Average % Removal		98.8%		99.5%		UOD lbs = (Effluent BOD lbs x 1.43) + (Effluent TKN lbs x 4.57)																				One Copy To The State With original Signature, Permittee To Keep One Copy On File						

Stowe Water Department - Demand & Capacity Analysis - EXHIBIT D

